

JCSD Secure Vestibules

Bridges High School
Madras, OR 97741

STRUCTURAL CALCULATIONS

Project Number: 10136.002

COLE Architects
1000 Wall St, Suite 205
Bend, OR 97703

Prepared by: Shawn Evilsizor PE, SE
3/8/2023

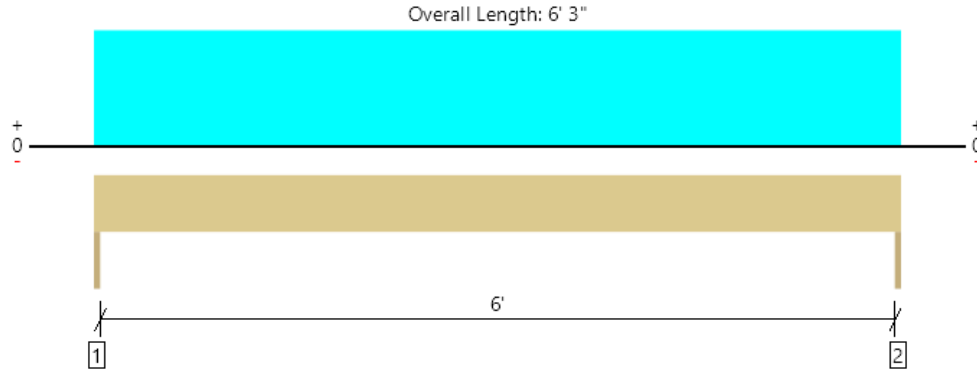
Bend-Redmond Office
2659 SW 4th Street Suite 102
Redmond, OR 97756
541.699.5432



EXPIRES: 12/31/2023



Level, Bridges (2) 2x10
2 piece(s) 2 x 10 DF No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1272 @ 0	2813 (1.50")	Passed (45%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	907 @ 10 3/4"	3830	Passed (24%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	1987 @ 3' 1 1/2"	4059	Passed (49%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.013 @ 3' 1 1/2"	0.125	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.044 @ 3' 1 1/2"	0.313	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/600) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	897	375	1272	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	897	375	1272	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	6' 3" o/c	
Bottom Edge (Lu)	6' 3" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 6' 3"	N/A	7.0	--	
1 - Uniform (PSF)	0 to 6' 3"	4'	20.0	30.0	Default Load
2 - Uniform (PSF)	0 to 6' 3"	10'	20.0	-	

Weyerhaeuser Notes

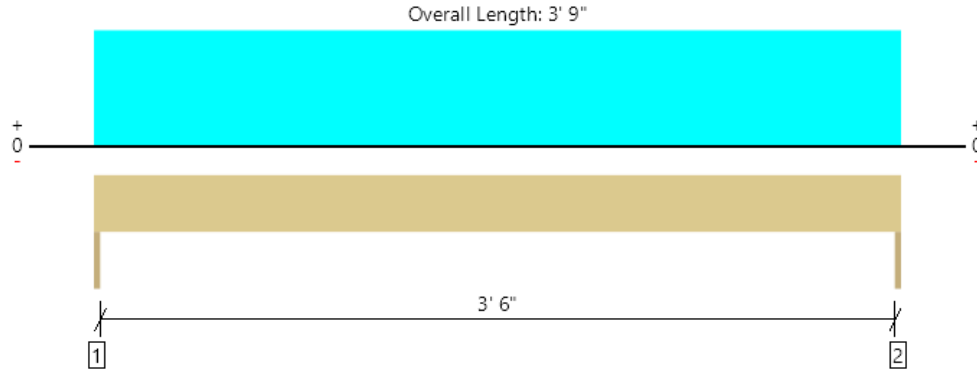
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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Eric Heidebrecht Morrison-Maierle (541) 330-6869 eheidebrecht@m-m.net	



Level, Bridges (2) 2x8
2 piece(s) 2 x 8 DF No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	760 @ 0	2813 (1.50")	Passed (27%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	465 @ 8 3/4"	3002	Passed (15%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	713 @ 1' 10 1/2"	2720	Passed (26%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.004 @ 1' 10 1/2"	0.075	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.012 @ 1' 10 1/2"	0.188	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/600) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	535	225	760	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	535	225	760	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	3' 9" o/c	
Bottom Edge (Lu)	3' 9" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 3' 9"	N/A	5.5	--	
1 - Uniform (PSF)	0 to 3' 9"	4'	20.0	30.0	Default Load
2 - Uniform (PSF)	0 to 3' 9"	10'	20.0	-	

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JCSD District Office
Madras, OR 97741

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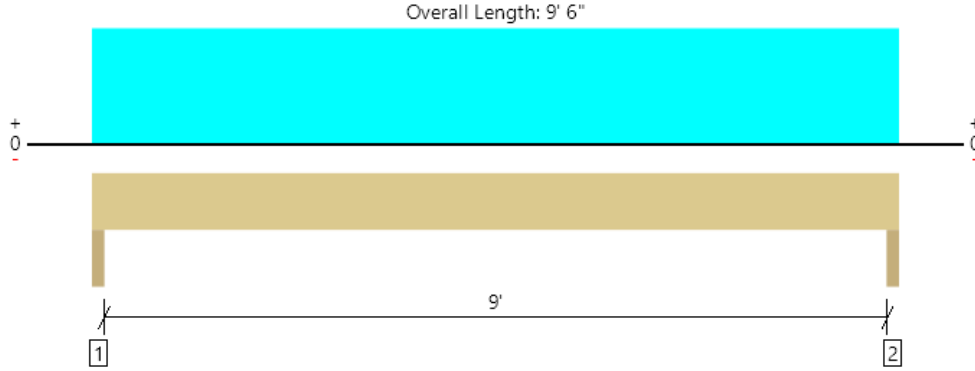
Bend-Redmond Office
2659 SW 4th Street Suite 102
Redmond, OR 97756
541.699.5432



EXPIRES: 12/31/2023



Level, District Office Entry Header
 1 piece(s) 3 1/8" x 12" 24F-V4 DF Glulam



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	5268 @ 1' 1/2"	6094 (3.00")	Passed (86%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	3882 @ 1' 3"	7619	Passed (51%)	1.15	1.0 D + 1.0 S (All Spans)
Pos Moment (Ft-lbs)	11862 @ 4' 9"	17250	Passed (69%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.122 @ 4' 9"	0.231	Passed (L/910)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.226 @ 4' 9"	0.308	Passed (L/492)	--	1.0 D + 1.0 S (All Spans)

System : Wall
 Member Type : Header
 Building Use : Residential
 Building Code : IBC 2018
 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/360).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Critical positive moment adjusted by a volume factor of 1.00 that was calculated using length L = 9' 3".
- The effects of positive or negative camber have not been accounted for when calculating deflection.
- The specified glulam is assumed to have its strong laminations at the bottom of the beam. Install with proper side up as indicated by the manufacturer.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Trimmer - SPF	3.00"	3.00"	2.59"	2418	2850	5268	None
2 - Trimmer - SPF	3.00"	3.00"	2.59"	2418	2850	5268	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	9' 6" o/c	
Bottom Edge (Lu)	9' 6" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 9' 6"	N/A	9.1	--	
1 - Uniform (PSF)	0 to 9' 6"	20'	25.0	30.0	Default Load

Weyerhaeuser Notes

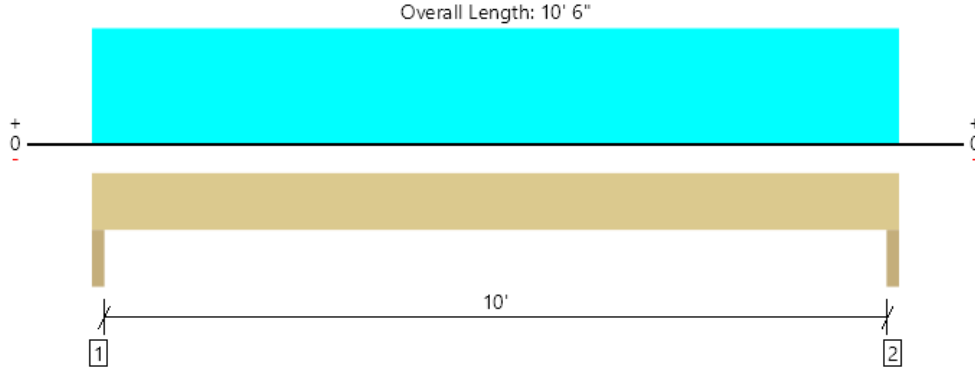
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ForteWEB Software Operator	Job Notes
Eric Heidebrecht Morrison-Maierle (541) 330-6869 eheidebrecht@m-m.net	



Level, District Office Office Header
 1 piece(s) 3 1/2" x 10 1/2" 24F-V4 DF Glulam



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	3354 @ 1' 1/2"	6825 (3.00")	Passed (49%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	2636 @ 1' 1 1/2"	7466	Passed (35%)	1.15	1.0 D + 1.0 S (All Spans)
Pos Moment (Ft-lbs)	8391 @ 5' 3"	14792	Passed (57%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.123 @ 5' 3"	0.256	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.261 @ 5' 3"	0.342	Passed (L/471)	--	1.0 D + 1.0 S (All Spans)

System : Wall
 Member Type : Header
 Building Use : Residential
 Building Code : IBC 2018
 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/360).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Critical positive moment adjusted by a volume factor of 1.00 that was calculated using length L = 10' 3".
- The effects of positive or negative camber have not been accounted for when calculating deflection.
- The specified glulam is assumed to have its strong laminations at the bottom of the beam. Install with proper side up as indicated by the manufacturer.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Trimmer - SPF	3.00"	3.00"	1.50"	1779	1575	3354	None
2 - Trimmer - SPF	3.00"	3.00"	1.50"	1779	1575	3354	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	10' 6" o/c	
Bottom Edge (Lu)	10' 6" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 10' 6"	N/A	8.9	--	
1 - Uniform (PSF)	0 to 10' 6"	10'	25.0	30.0	Default Load
2 - Uniform (PSF)	0 to 10' 6"	8'	10.0	-	

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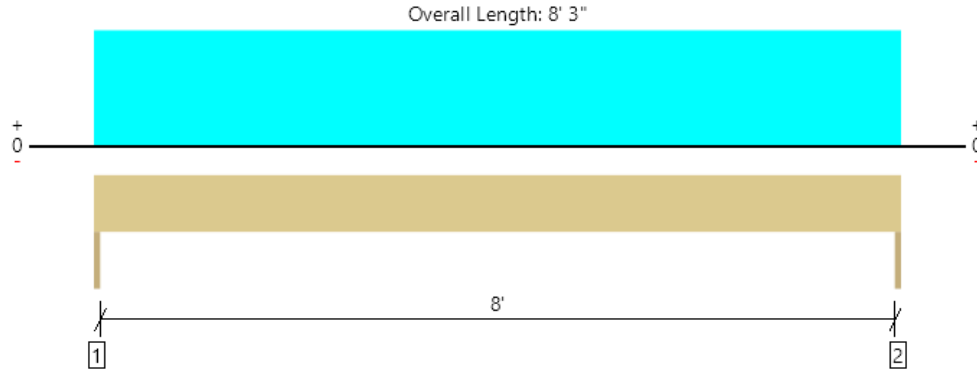
Bend-Redmond Office
2659 SW 4th Street Suite 102
Redmond, OR 97756
541.699.5432



EXPIRES: 12/31/2023



Level, Madras High School Header
1 piece(s) 6 x 8 DF No.1



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1157 @ 0	5156 (1.50")	Passed (22%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	947 @ 9"	5376	Passed (18%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	2386 @ 4' 1 1/2"	5930	Passed (40%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.030 @ 4' 1 1/2"	0.165	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.094 @ 4' 1 1/2"	0.412	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/600) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	786	371	1157	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	786	371	1157	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	8' 3" o/c	
Bottom Edge (Lu)	8' 3" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 8' 3"	N/A	10.4	--	
1 - Uniform (PSF)	0 to 8' 3"	3'	20.0	30.0	Default Load
2 - Uniform (PSF)	0 to 8' 3"	6'	20.0	-	

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3/8/2023

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EXPIRES: 12/31/2023



Project Title:
 Engineer:
 Project ID:
 Project Descr:

Steel Beam

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Lic. # : KW-06000001

DESCRIPTION: --None--

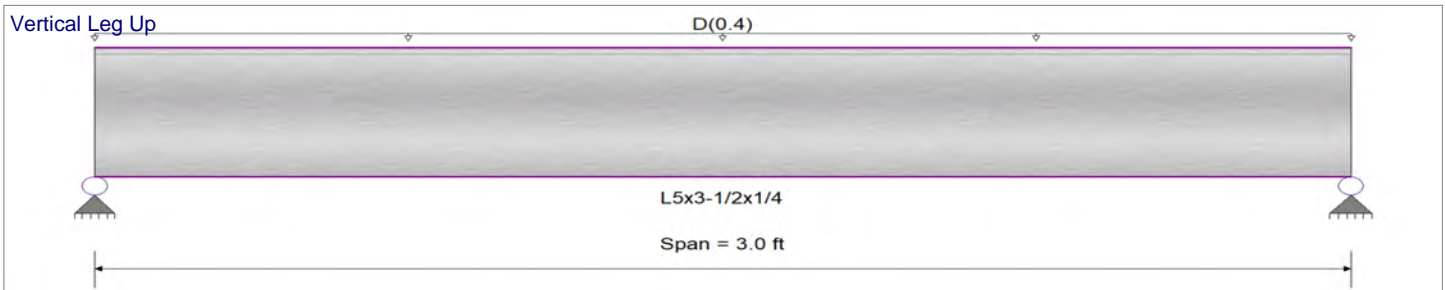
CODE REFERENCES

Calculations per
 Load Combination Set : ASCE 7-16

Material Properties

Analysis Method : Load Resistance Factor Design
 Beam Bracing : Beam is Fully Braced against lateral-torsional buckling
 Bending Axis : Major Axis Bending

Fy : Steel Yield : 36.0 ksi
 E: Modulus : 29,000.0 ksi



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading
 Uniform Load : D = 0.40 k/ft, Tributary Width = 1.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	0.125 : 1	Maximum Shear Stress Ratio =	0.035 : 1
Section used for this span	L5x3-1/2x1/4	Section used for this span	L5x3-1/2x1/4
Mu : Applied	0.641 k-ft	Vu : Applied	0.8547 k
Mn * Phi : Allowable	5.109 k-ft	Vn * Phi : Allowable	24.30 k
Load Combination	+1.40D+1.60H	Load Combination	+1.40D+1.60H
Location of maximum on span	1.500ft	Location of maximum on span	0.000 ft
Span # where maximum occurs	Span # 1	Span # where maximum occurs	Span # 1
Maximum Deflection			
Max Downward Transient Deflection	0.000 in	Ratio =	0 < 360
Max Upward Transient Deflection	0.000 in	Ratio =	0 < 360
Max Downward Total Deflection	0.005 in	Ratio =	7510 >= 600
Max Upward Total Deflection	0.000 in	Ratio =	0 < 600

Maximum Forces & Stresses for Load Combinations

Load Combination	Segment Length	Span #	Max Stress Ratios		Summary of Moment Values					Summary of Shear Values				
			M	V	max Mu +	max Mu -	Mu Max	Mnx	Phi*Mnx	Cb	Rm	VuMax	Vnx	Phi*Vnx
+1.40D+1.60H Dsgn. L = 3.00 ft		1	0.125	0.035	0.64		0.64	5.68	5.11	1.00	1.00	0.85	27.00	24.30
+1.20D+0.50Lr+1.60L+1.60H Dsgn. L = 3.00 ft		1	0.108	0.030	0.55		0.55	5.68	5.11	1.00	1.00	0.73	27.00	24.30
+1.20D+1.60L+0.50S+1.60H Dsgn. L = 3.00 ft		1	0.108	0.030	0.55		0.55	5.68	5.11	1.00	1.00	0.73	27.00	24.30
+1.20D+1.60Lr+L+1.60H Dsgn. L = 3.00 ft		1	0.108	0.030	0.55		0.55	5.68	5.11	1.00	1.00	0.73	27.00	24.30
+1.20D+1.60Lr+0.50W+1.60H Dsgn. L = 3.00 ft		1	0.108	0.030	0.55		0.55	5.68	5.11	1.00	1.00	0.73	27.00	24.30
+1.20D+L+1.60S+1.60H Dsgn. L = 3.00 ft		1	0.108	0.030	0.55		0.55	5.68	5.11	1.00	1.00	0.73	27.00	24.30
+1.20D+1.60S+0.50W+1.60H Dsgn. L = 3.00 ft		1	0.108	0.030	0.55		0.55	5.68	5.11	1.00	1.00	0.73	27.00	24.30
+1.20D+0.50Lr+L+W+1.60H Dsgn. L = 3.00 ft		1	0.108	0.030	0.55		0.55	5.68	5.11	1.00	1.00	0.73	27.00	24.30
+1.20D+L+0.50S+W+1.60H Dsgn. L = 3.00 ft		1	0.108	0.030	0.55		0.55	5.68	5.11	1.00	1.00	0.73	27.00	24.30
+0.90D+W+1.60H Dsgn. L = 3.00 ft		1	0.081	0.023	0.41		0.41	5.68	5.11	1.00	1.00	0.55	27.00	24.30
+1.20D+L+0.20S+E+1.90H Dsgn. L = 3.00 ft		1	0.108	0.030	0.55		0.55	5.68	5.11	1.00	1.00	0.73	27.00	24.30
+0.90D+E+0.90H Dsgn. L = 3.00 ft		1	0.081	0.023	0.41		0.41	5.68	5.11	1.00	1.00	0.55	27.00	24.30

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Steel Beam

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DESCRIPTION: --None--

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
D Only	1	0.0048	1.509		0.0000	0.000

Maximum Deflections for Load Combinations

Load Combination	Span	Max. Downward Defl	Location in Span	Span	Max. Upward Defl	Location in Span
+D+H	1	0.0048	1.509		0.0000	0.000
+D+L+H	1	0.0048	1.509		0.0000	0.000
+D+Lr+H	1	0.0048	1.509		0.0000	0.000
+D+S+H	1	0.0048	1.509		0.0000	0.000
+D+0.750Lr+0.750L+H	1	0.0048	1.509		0.0000	0.000
+D+0.750L+0.750S+H	1	0.0048	1.509		0.0000	0.000
+D+0.60W+H	1	0.0048	1.509		0.0000	0.000
+D+0.750Lr+0.450W+H	1	0.0048	1.509		0.0000	0.000
+D+0.750S+0.450W+H	1	0.0048	1.509		0.0000	0.000
+0.60D+0.60W+0.60H	1	0.0029	1.509		0.0000	0.000
+D+0.70E+0.60H	1	0.0048	1.509		0.0000	0.000
+D+0.750L+0.750S+0.5250E+H	1	0.0048	1.509		0.0000	0.000
+0.60D+0.70E+H	1	0.0029	1.509		0.0000	0.000
D Only	1	0.0048	1.509		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1

Values in KIPS

Load Combination	Support 1	Support 2
Overall MAXimum	0.611	0.611
Overall MINimum	0.366	0.366
+D+H	0.611	0.611
+D+L+H	0.611	0.611
+D+Lr+H	0.611	0.611
+D+S+H	0.611	0.611
+D+0.750Lr+0.750L+H	0.611	0.611
+D+0.750L+0.750S+H	0.611	0.611
+D+0.60W+H	0.611	0.611
+D+0.750Lr+0.450W+H	0.611	0.611
+D+0.750S+0.450W+H	0.611	0.611
+0.60D+0.60W+0.60H	0.366	0.366
+D+0.70E+0.60H	0.611	0.611
+D+0.750L+0.750S+0.5250E+H	0.611	0.611
+0.60D+0.70E+H	0.366	0.366
D Only	0.611	0.611
Lr Only		
L Only		
S Only		
W Only		
E Only		
H Only		